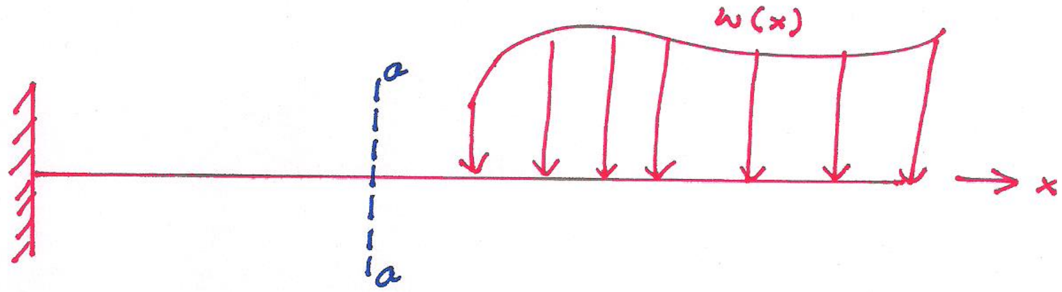
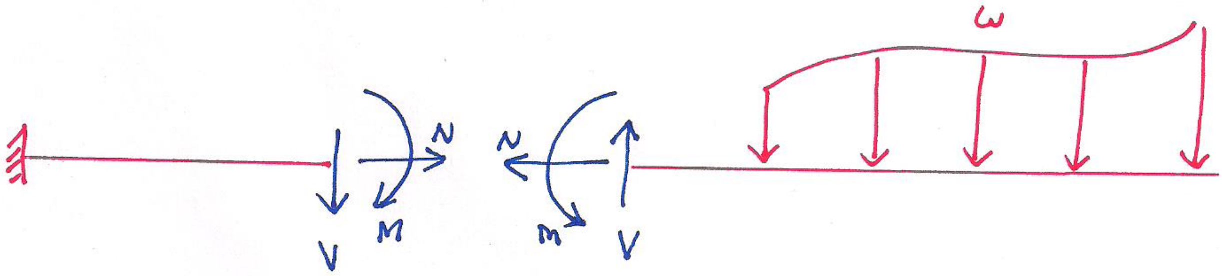
Flexural Systems
"Beams and Frames"

When a beam or frame is subjected to transverse loadings, the three possible internal forces that are developed are the normal or axial force, the shearing force, and the bending moment, as shown in section a-a of the cantilever beam.

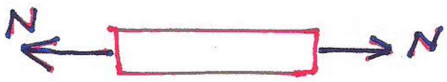




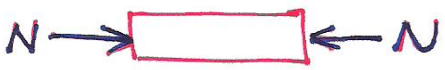
Internal forces in a beam

To predict the behavior of structures, the magnitudes of these forces must be known.

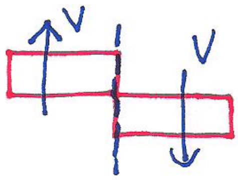
Sign Convention



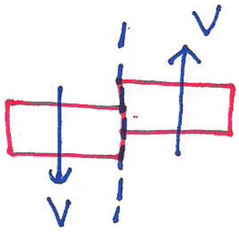
Positive axial force "Tension"



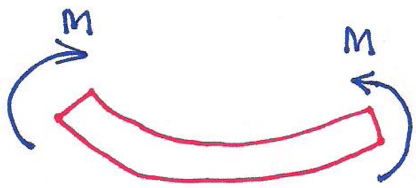
Negative axial force "Compression"



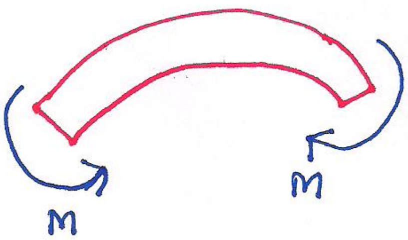
Positive shear force



Negative shear force



Positive bending moment



Negative bending moment

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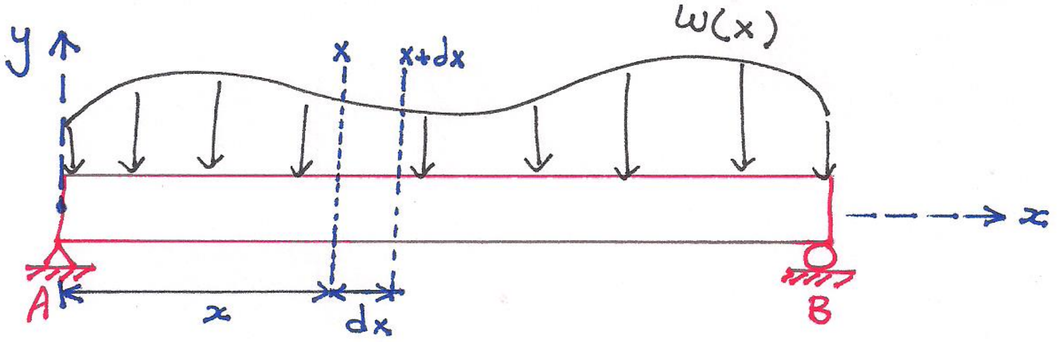
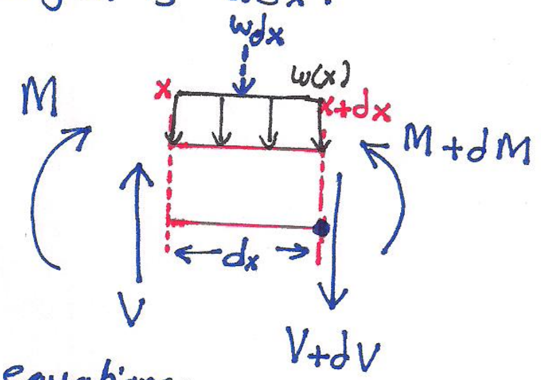
Relation Among distributed load, shearing force, and bending moment

Consider the simply supported beam (AB) which is subjected to the load $w(x)$



Let the shear force and bending moment at a section located at a distance of x from the left support be V and M, respectively, and at a section $x+dx$ be $V+dV$ and $M+dM$, respectively. The total load acting through the center of the infinitesimal length is $w\,dx$.



* Apply positive sign convention

Apply the equilibrium equations:

$$+\uparrow \Sigma F_y = 0: \quad +V - w.dx - (V + dV) = 0 \rightarrow \frac{dV}{dx} = -w$$

$$\Delta V = -\int w\,dx$$

$$\circlearrowleft + \Sigma M_{\substack{\text{right}\\ \text{corner}}} = 0: \quad +(M+dM) + w\,dx\left(\frac{dx}{2}\right) - V.dx - M = 0$$

$$\rightarrow \frac{dM}{dx} = V$$

$$\Delta M = \int V\,dx$$

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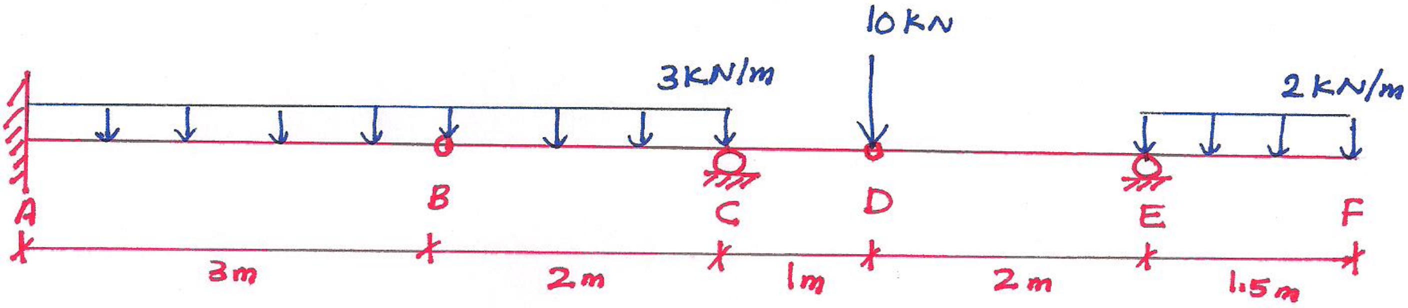
Example: Shear force "SFD" and bending moment diagrams "BMD"

Beam AF has a fixed support at A, internal hinges at B and D, rollers at C and E, and free end F. It is subjected to the transverse loadings shown. Determine the reactions and draw SFD & BMD.



Solution

The beam is statically determinate because the number of unknown reactions = number of equilibrium equations

No. of reactions = 9

No. of equations = 9

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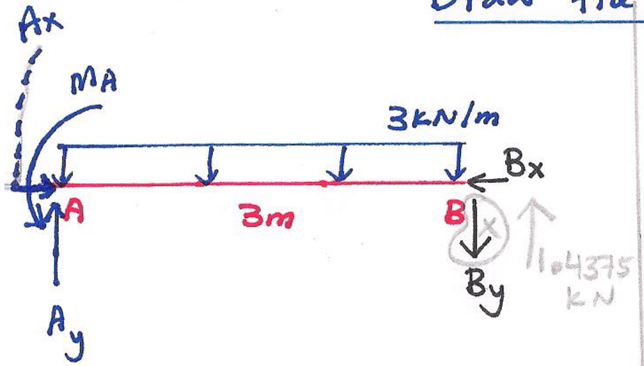
Draw the FBD and determine the reactions



$\overset{+}{\circlearrowleft}\Sigma M_A = 0$

$+M_A + 1.4375(3) - 3(3)(1.5) = 0$

$M_A = +9.1875$ kN.m

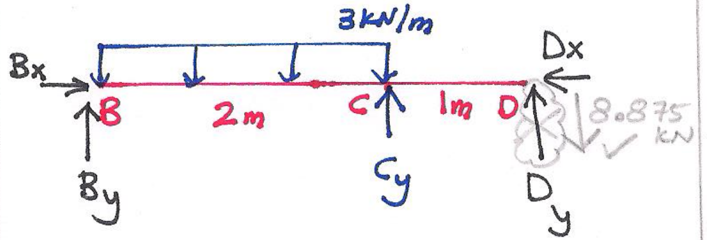
$+\uparrow \Sigma F_y = 0$

$+A_y - 3(3) + 1.4375 = 0$

$A_y = +7.5625$ kN

$\Sigma F_x = 0$

$A_x = 0$



$\overset{+}{\circlearrowleft}\Sigma M_B = 0$

$-3(2)(1) + 2C_y - 3(8.875) = 0$

$C_y = 16.3125$ kN

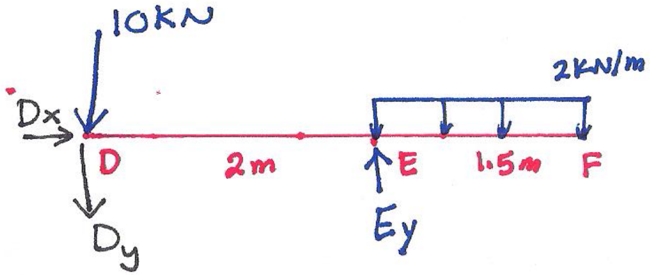
$+\uparrow \Sigma F_y = 0$

$+B_y - 3(2) + 16.3125 - 8.875 = 0$

$B_y = -1.4375$ kN $(\downarrow)$

$\Sigma F_x = 0$

$B_x = 0$



Only 3-unknowns → start from here.

$\overset{+}{\circlearrowleft}\Sigma M_D = 0 : +2E_y - 2(1.5)(2.75) = 0$

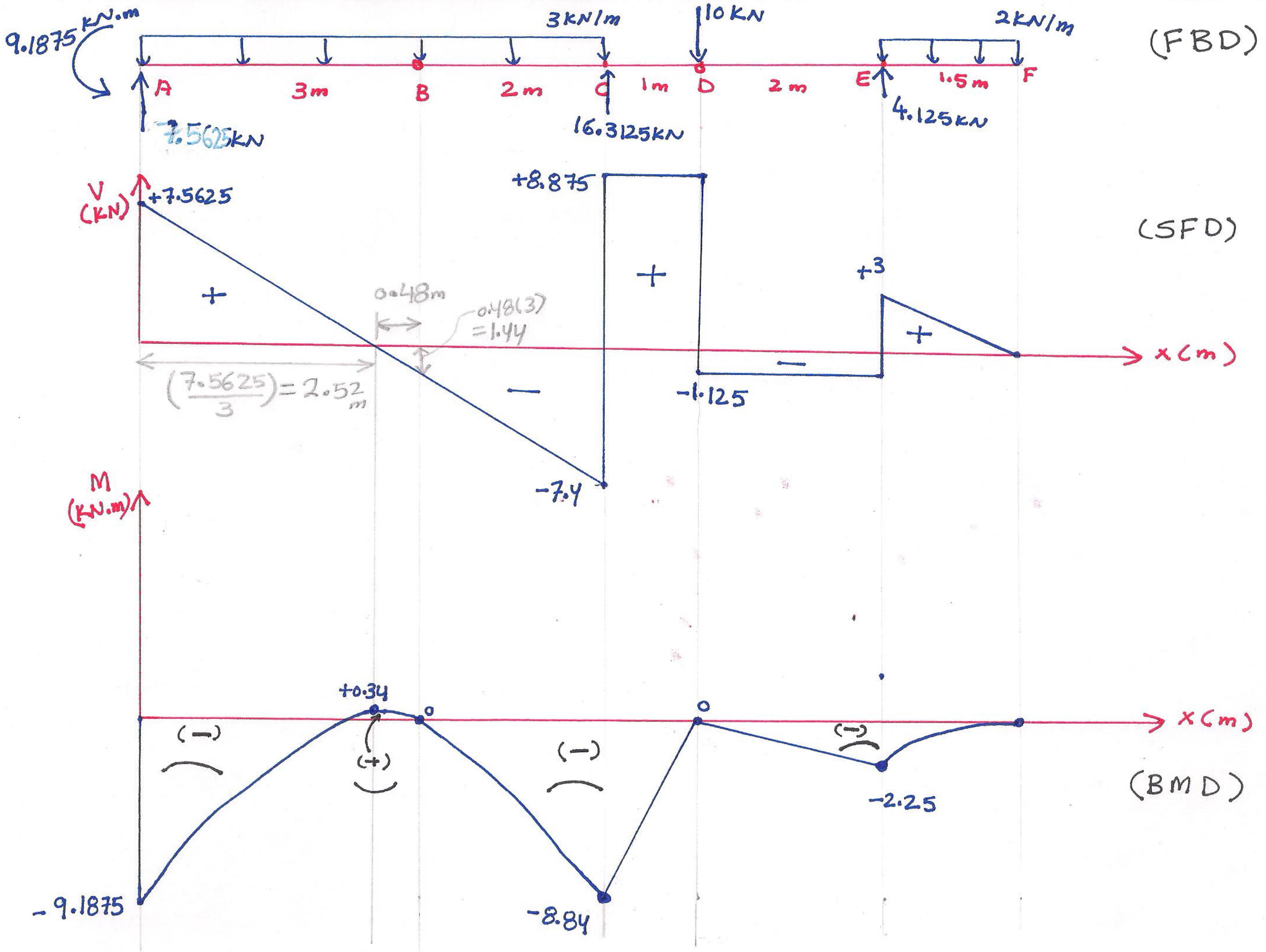
$E_y = 4.125$ kN $(\uparrow)$

$+\uparrow \Sigma F_y = 0$

$-10 - D_y + 4.125 - 2(1.5) = 0$

$D_y = -8.875$ kN $(\uparrow)$

$\Sigma F_x = 0 : \quad D_x = 0$



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Write the equations of shear force and bending moment

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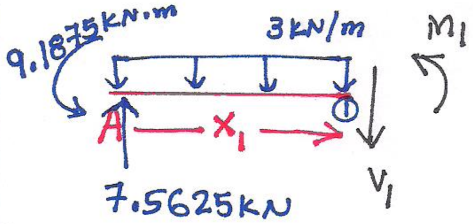
* Part ABC:



$0 \le x_1 \le 5m$

$+\uparrow \Sigma F_y = 0: \; +7.5625 - 3x_1 - V_1 = 0$

$V_1 = 7.5625 - 3x_1$

$\circlearrowleft + \Sigma M_{1} = 0: \; +M_1 + 3x_1 (x_1/2) + 9.1875 - 7.5625\, x_1 = 0$

$M_1 = -1.5\, x_1^2 + 7.5625\, x_1 - 9.1875$

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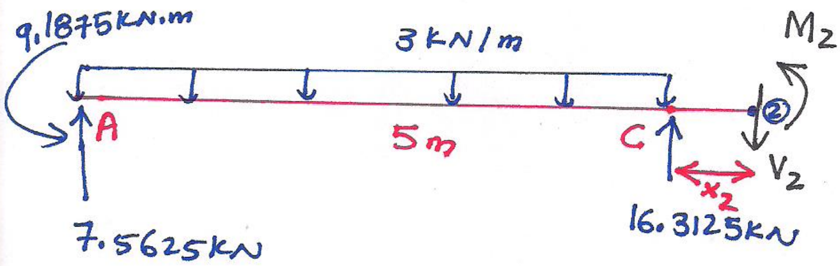
* Part CD: $0 \le x_2 \le 1m$



$+\uparrow \Sigma F_y = 0$

$+7.5625 - 3(5) + 16.3125 - V_2 = 0$

$V_2 = +8.875 \text{ kN}$

$\circlearrowleft + \Sigma M_{2} = 0: \; + M_2 - 16.3125(x_2) + 3(5)(x_2 + \frac{5}{2}) + 9.1875 - 7.5625(5 + x_2) = 0$

$M_2 = +8.875\, x_2 - 8.875$

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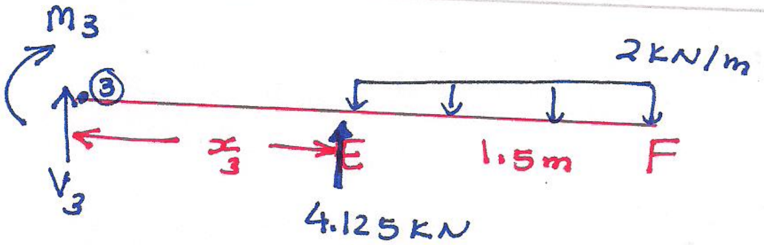
* Part DE:



$0 \le x_3 \le 2m$

$+\uparrow \Sigma F_y = 0: \; +V_3 + 4.125 - 2(1.5) = 0 \rightarrow V_3 = -1.125 \text{ kN}$

$\circlearrowleft + \Sigma M_{3} = 0: \; -M_3 + 4.125\, x_3 - 2(1.5)(x_3 + 1.5/2) = 0$

$M_3 = +1.125\, x_3 - 2.25$

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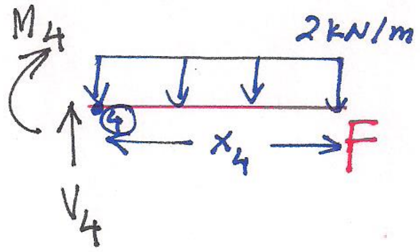
* Part EF :



$0 \leq x_4 \leq 1.5\,m$

$+\uparrow \sum F_y = 0 : \quad +V_4 - 2x_4 = 0 \rightarrow V_4 = +2x_4$

$\circlearrowleft + \sum M_{④} = 0 : \quad -M_4 - 2x_4 \left(x_4/2 \right) = 0$

$M_4 = -x_4^2$

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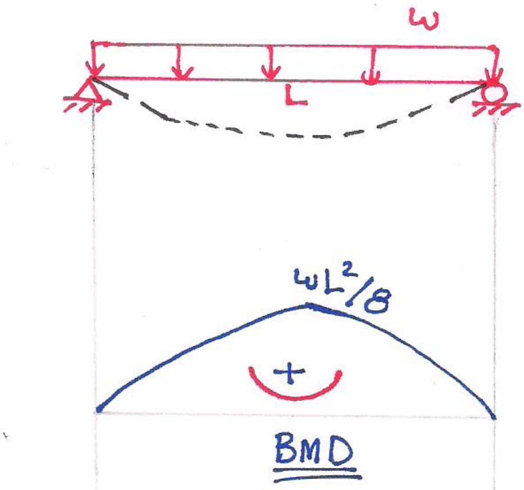
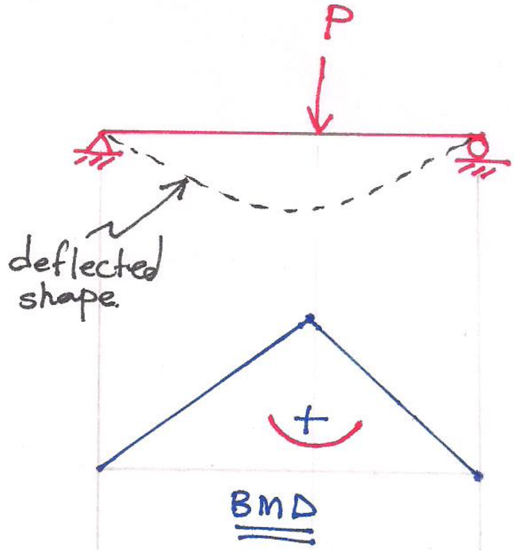
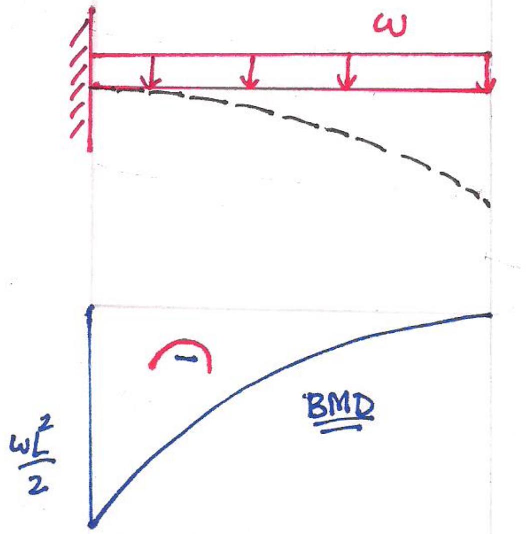
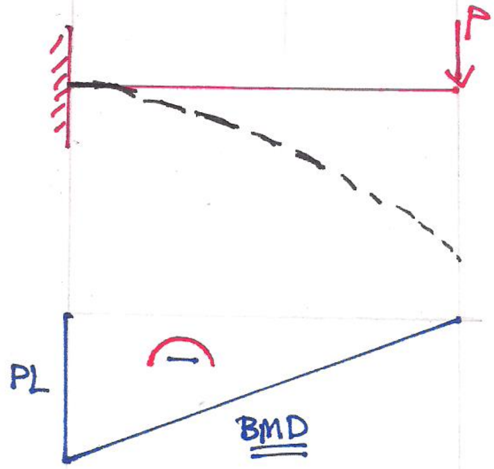
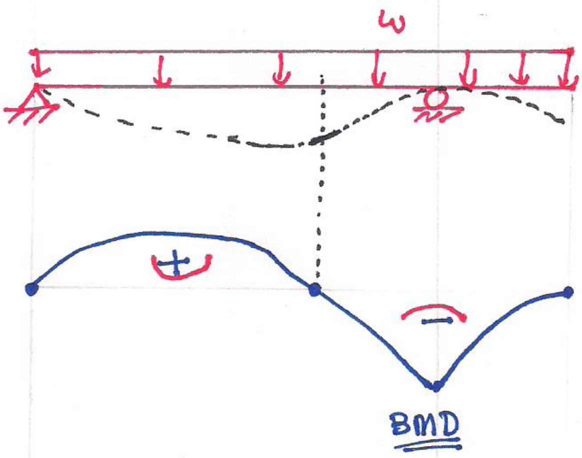
Deformed shape "Qualitative"

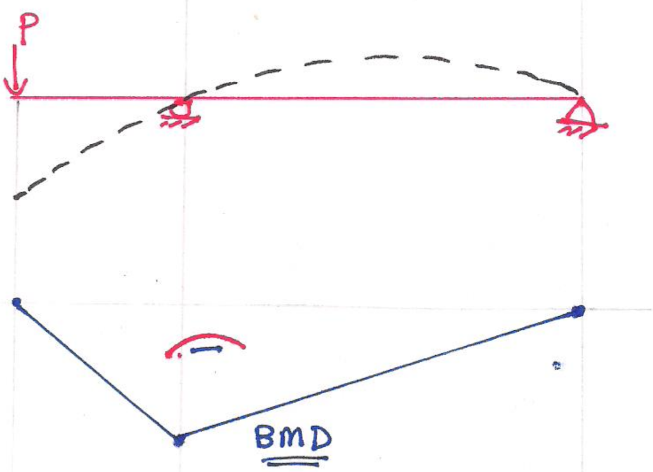












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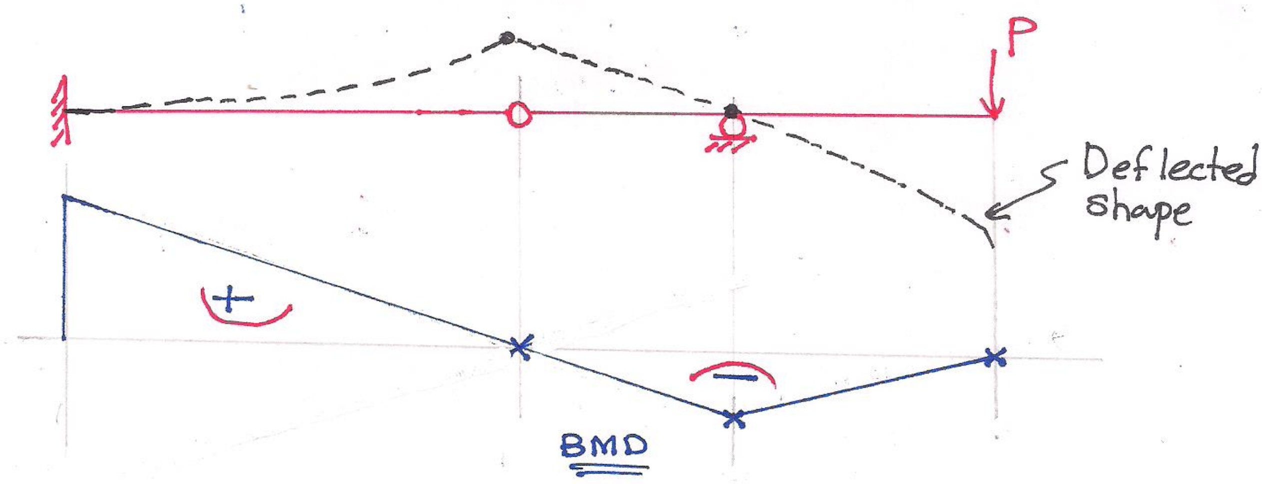

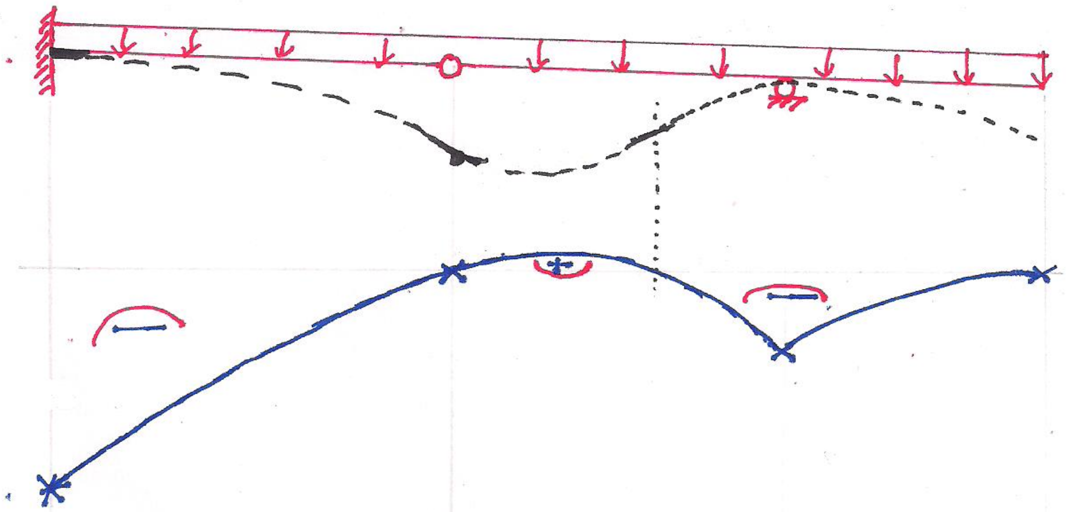

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