

Design

Known Beam dimensions



calculate the required reinforcement

Unknown Beam dimensions

known M_u
set the reinforcement ratio ρ, A, B, H

set the reinforcement ratio

Economic reinforcement $0.5 \rho_{max} \leq \rho \leq 0.75 \rho_{max}$

Force Equilibrium and Nominal Strength

Reinforcement ratio and Nominal Strength

$$a = \frac{A_s f_y}{0.85 f_c' B}$$

$$M_u = \phi M_n = \phi A_s f_y \left(d - \frac{a}{2} \right)$$

بذل a لا نعرفه $\rightarrow$ assumption $a \approx \frac{d}{3}$

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$$a \approx \frac{d}{3}$$

بذل a لا نعرفه $\rightarrow$ Check a

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$$M_u = \phi M_n = \phi \rho f_y B d^2 \left(1 - 0.59 \rho \frac{f_y}{f_c'} \right)$$

بذل d لا نعرفه $\rightarrow$ Find an adequate section (Bro)

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Using design Aids on ITC

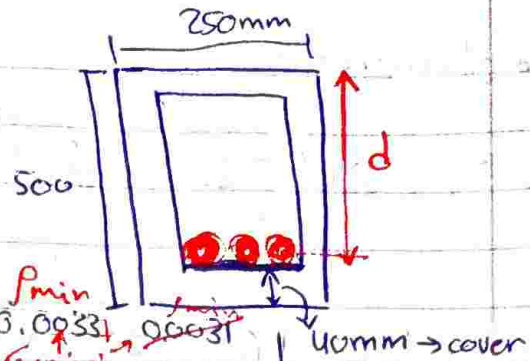
• Example: $M_u = 120 \text{ kN.m}$

$f_c' = 28 \text{ MPa}$

$f_y = 420 \text{ MPa}$

Design Aids: TABLE A.4

f_c'	B_1	$\rho_{0.005}$	$\rho_{0.004}$	ρ_{min}
28	0.85	0.0181	0.0206	0.00331



Assume $d = 16 \text{ mm}$
 depth channel = 10mm
 $\phi = 0.9$

• y_d : $40 + 10 + 8 = 58 \text{ mm}$

• $d = H - y_d = 500 - 58 = 442 \text{ mm}$

① iterations: $a \rightarrow A_s \rightarrow \text{Check } a$
 $\text{assume } a = \frac{d}{3} = \frac{442}{3} \approx 147 \text{ mm}$

① $M_u = \phi A_s f_y (d - \frac{a}{2}) \Rightarrow 120 \times 10^6 = 0.9 A_s 420 (442 - \frac{147}{2})$
 $A_s = 853.3 \text{ mm}^2$

• check a : $a = \frac{A_s f_y}{0.85 f_c' B} = \frac{853.3 \times 420}{0.85 \times 28 \times 250} = 60.2 \text{ mm}$

② $a = 80 \text{ mm}$
 $M_u = \phi A_s f_y (d - \frac{a}{2}) \Rightarrow A_s = 789.7 \text{ mm}^2$

• check a : $a = \frac{A_s f_y}{0.85 f_c' B} = 55.7 \text{ mm}$

③ $a = 55 \text{ mm}$
 $A_s = 766 \text{ mm}^2 \rightarrow$ $a = 54 \text{ mm}$
 (بعضه هاي) $\rightarrow$ $a = 54 \text{ mm}$
 (بعضه هاي) $\rightarrow$ $a = 54 \text{ mm}$
 « 0.03 بغير فرق »

بهره، مثال، افزون ربا
 معنا، و لكن عا
 عايزيد، الليوسنة
 اويجيد، و بيجل
 Check $\rightarrow$ معنا

$A_s = 766 \rightarrow$

Design Aids

Table A.2

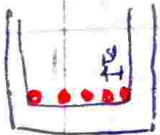
بیشتر از اهاي الخيارات بتوسع
 بالبر

24	22
34	16
36	13

أقرب قيم $\leftarrow$

• options:-

$\Rightarrow 2\phi 22$ ✓ $\Rightarrow \rho = 250 - 20 - 44 - 80 = 106 > 25 \text{ mm}$
 $\rho = 106$ spaces 

$\Rightarrow 4\phi 16$ ✓ $\Rightarrow 3\phi \Rightarrow 250 = 80 + 20 + 4 \times 16 + 3\phi$
 $\rho = 28.6 > 25 \text{ mm}$ 

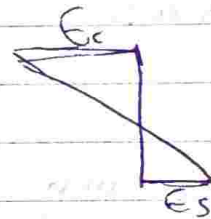
$\Rightarrow 6\phi 13$ ✗ $\Rightarrow 5\phi \Rightarrow 250 = 80 + 20 + 6 \times 13 + 5\phi$
 $\rho = 14.4 < 25 \text{ mm}$

Design Aids
 → Table A.7

بدل صاف ممكن ارجع

• Check on ϕ :

$0.004 < \rho < 0.005$



$\rho = \frac{A_s}{Bd} = \frac{796}{250 \times 442} = 0.0072 < \rho_{0.005}$ ✓
 $\rho_{0.005}$ is ρ_{max}
 $> \rho_{min}$

• Check on ϕM_n :-

$a = \frac{A_s f_y}{0.85 f'_c b} = \frac{796 \times 420}{0.85 \times 28 \times 442} = 56.18 \text{ mm}$

$\phi M_n = \phi A_s f_y (d - \frac{a}{2})$
 $= 0.9 \times 796 \times 420 (442 - \frac{56.18}{2})$
 $= 124.5 \text{ kN.m} > M_u$ ✓

Note: $A_s \neq n \times \frac{\pi}{4} (d)^2$

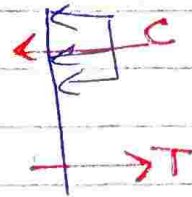
لا نه يكون في ممرات في البارز عباتر عتمة اياها للبارز
 القيم الي تكون بالمقابل تكون مراعية لها الفرقه الفرقه صغيره

* Method 2:

$$\Rightarrow T = C$$

$$\bullet a = \frac{A_s f_y}{0.85 f'_c b}$$

$$\bullet \phi M_n = \phi A_s f_y \left(d - \frac{a}{2} \right)$$



$$\Rightarrow M_u \leq \phi M_n = \phi \rho f_y b d^2 \left(1 - 0.59 \rho \frac{f_y}{f'_c} \right)$$

$$\Rightarrow \frac{M_u}{\phi b d^2} = \rho \frac{f_y}{f'_c} \left(1 - 0.59 \rho \frac{f_y}{f'_c} \right)$$

الطريقة الأولى

TABLE A.4

ρ	$\rho_{0.005}$ $\rho_{0.005}$	$\rho_{0.004}$ $\rho_{0.004}$	$\rho_{min} = \frac{1.4}{f_y}$	$\rho_{mi} = \frac{0.85 f'_c}{f_y}$
0.25	0.0181	0.0206	0.0033	0.0031

$$= 2.73 \text{ MPa}$$

نظرات التي هي فيها
يعني متوسطي
أخذت بوضع عليها

Table A.5a

$$\rho = 0.0065$$

$$\rho = 0.007$$

$$f_y = 420 \text{ MPa}$$

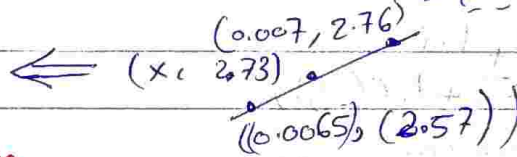
$$f'_c = 28$$

$$2.57$$

$$2.76$$

نظر انتر بولين

$$\bullet \frac{2.76 - 2.57}{0.007 - 0.0065} = \frac{2.73 - 2.57}{x - 0.0065}$$



$$x = \rho = 0.00692 > \rho_{min}$$

$$< \rho_{max}$$

$$\bullet A_s = \rho b d = 764.7 \text{ mm}^2$$

$$\bullet 4 \phi 16 \rightarrow A_s = 796 \text{ mm}^2$$

$$\bullet \rho = 0.0072$$

Table A.5a

$$\rho = 0.007$$

$$\rho = 0.0075$$

$$f_y = 420 \text{ MPa}$$

$$f'_c = 28$$

$$2.76$$

$$2.94$$

$$R = 2.844 \text{ MPa}$$

نخرج في الجداول
ونطول الباريات وقطرها

Table A.7

$$R = \frac{M_u}{\phi b d^2} = 2.844 \times 9 \times 250 \times (442)^2 = 125 \text{ Kib.m}$$

$$\phi M_n = 125 > M_u \text{ OK}$$

→ Example: simply supported Beam

1 span → 4.5 m

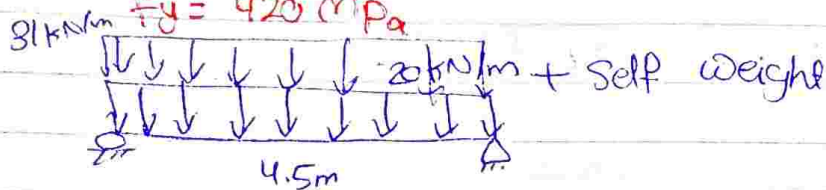
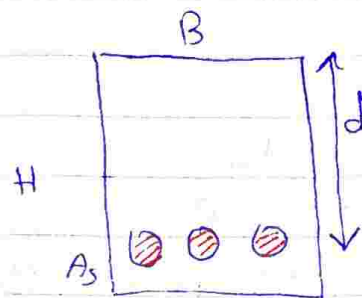
dead load → 20 kN/m

live load → 31 kN/m

$$\rho = 0.5 \rho_{max}$$

$$f_c = 28 \text{ MPa}$$

$$f_y = 420 \text{ MPa}$$



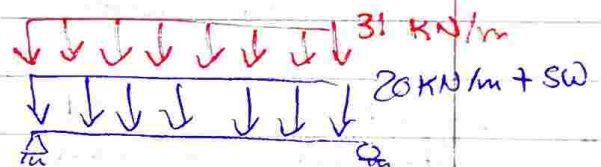
1) $M_u = ?$

assume

Initial assumptions.
 • [Table 9.3.1.1] Minimum depth $(H) = \frac{L}{16} = \frac{4.5}{16} = 0.3 \text{ m} = 300 \text{ mm}$
 $B = 200 \text{ mm}$

• $SW = 1 \times 0.3 \times 0.2 \times \frac{24 \text{ kN/m}^3}{24 \text{ kN/m}^3} = 1.44 = 1.5$

• $W_u = 1.2 D + 1.6 L$
 $= 1.2(21.5) + 1.6(31)$
 $= 75.4 \text{ kN/m}$



• Simply supported Beam → $M_u = \frac{W_u L^2}{8} = \frac{(75.4)(4.5)^2}{8} = 191 \text{ kN.m}$

• $\rho = 0.5 \rho_{max} = 0.5 \times 0.0206 = 0.0103$

TABLE A.4

Method 2 : $\rho = 0.0103 \rightarrow R = ??$

$R = 3.932 \text{ MPa}$

$R = \frac{M_u}{bd^2} \Rightarrow bd^2 = \frac{191 \text{ kN.m}}{3.932 \text{ MPa}} = 48.5 \times 10^6 \text{ mm}^3$

ϕbd^2

→ If $\rho < \rho_{0.005} \Rightarrow \phi = 0.9$

ρ	R
0.01	3.83
0.0105	4

3.93 MPa

- mostly economical $\rightarrow d = (1.5-3)b$

٥. $\frac{d}{b}$ تقريبا $1.5 \leftarrow$ بصر الجسيم
 ضيق، ما نوع لبارات مناسبة
 $d > b$

b mm	d mm	
200	520	→ factor
300	425	→ 1.5
400	365	ok
	347	
	347	
250	465	yellow

- o B $\rightarrow$ 250 mm

- $d \Rightarrow 465 \text{ mm} \rightarrow H = 465 + 40 + 10 + 10 = 525$

• Check S.W =

↓ ↓ ↓ ↓
d cover stress diameter
of bar
mm بقرها الأقرب

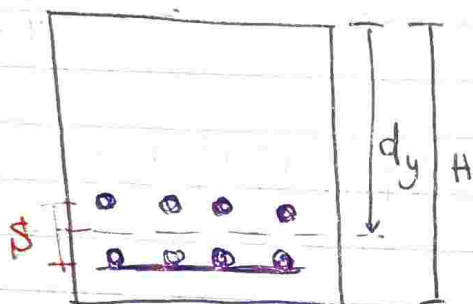
set to $\Rightarrow B = 250 \text{ mm}$

$$H = 550 \text{ mm}$$

S.W = 3.4 kN/m

$$\bullet \quad \mu_u = \frac{(77.7) \times (4.5)^2}{8} = 197 \text{ kN.m}$$

- positive moment $\rightarrow$ Bottom reinforcement
- negative moment $\rightarrow$ Top reinforcement



مع $\frac{1}{2} \phi$ من ϕ في ϕ .

$$d_y = H - \left(\text{cover} + \text{straps} + d + \frac{1}{2} \phi \right)$$