Asphalt binders & asphalt mixtures

Chapter 13

Bituminous Materials

- Definition: Bituminous materials are <u>one of the oldest</u> materials used in construction (Asphalt binders were used in 3000 B.C.). It is a solid, semisolid, or viscous cementitious material, natural or manufactured, and composed of "hydrocarbons".
- The most common bituminous materials are
 - Asphalts : available as natural deposits or are produced from petroleum processing.
 - Tars : obtained through the destructive distillation of materials such as wood, and coal.



Engineering use of bituminous materials

- Asphalt is used mostly in pavement construction, but is also used as sealing and waterproofing agents.
- Tar is used primarily for waterproofing membranes. Tar may also be used for pavement treatments.



Based on its source, Asphalt can be categorized as:

- Natural Rock asphalts: obtained from rock deposits containing bituminous materials.
- Native asphalts: obtained from asphalt lakes in Trinidad and other Caribbean areas.
- Petroleum asphalts: products of the distillation of crude oil. These asphalts are used as the most common bituminous paving materials.





The main advantages of asphalt are:

- Asphalt is strong and durable cementing material with excellent adhesive and waterproofing characteristics.
- Cost-Effective. Widely available material; fast to construct; and provides for smooth roads increasing traffic safety and reducing wear and tear on vehicles which brings down vehicle operating costs.
- Environmentally friendly. Asphalt is 100% recyclable and the most recycled product in America.
- Highly resistant to the action of most acids, alkalis and salts.

Chemical Composition of Asphalt

- Asphalt is a mixture of a wide variety of hydrocarbons of different molecular weights primarily consisting of hydrogen and carbon, with minor components such as sulfur, nitrogen, and oxygen and trace metals.
- In the compounds level asphalt consists of two major compounds:
 - Asphaltenes and
 - Maltenes (petrolenes) which consist of resins and oils.



Chemical Composition of Asphalt

- The asphaltenes are dark brown, friable solids responsible for the <u>viscosity and the adhesive property of the asphalt</u>. If the asphaltene content is less than 10%, the asphalt concrete will be difficult to compact to the proper construction density.
- The maltenes (petrolenes) consist of resins and oils.
 - Resins are dark and semisolid or solid, with a viscosity that is largely affected by temperature. The resins act as agents to disperse asphaltenes in the oils.
 - Oils are clear or white liquids.
- The physical and aging properties of asphalt depend on its chemical composition and molecular structure. As asphalt includes heavier molecules it becomes harder and more viscous. Additionally, asphalt sensitivity to temperature is also dependent on its chemical composition.

Temperature Susceptibility of Asphalt

- At room temperature Asphalt is a semi solid material. The consistency of asphalt is greatly affected by temperature. Asphalt gets hard and brittle at low temperatures and soft at high temperatures.
- Asphalt's temperature sensitivity can be represented by the slope of the line shown in the Figure; the steeper the slope the higher the temperature susceptibility (sensitivity) of the asphalt.



Temperature Susceptibility of Asphalt

 Due to temperature susceptibility, the grade of the asphalt cement should be selected according to the climate of the area. The viscosity of the asphalt should be mostly within the optimum range for the area's annual temperature range; soft grade asphalts are used for cold climates and hard-grade asphalts for hot climates



Selecting the proper grade of asphalt binder to match the climate.

Aging or Hardening of Asphalt

Aging or hardening is the process under which asphalt becomes harder and brittle due to increase in the viscosity of asphalt caused by:

- The evaporation and oxidation of the lighter, oily constituents during mixing at high temperatures, called "volatilization" (short-term aging or hardening).
- 2. The oxidation of the oils to resins and resins to asphaltenes when used over a period of years (longterm or in-service aging or hardening)



Types of Asphalt Products

Asphalt used in pavements is produced in three forms:

- Asphalt cement
- Asphalt cutback, and
- Asphalt emulsion.



Natural Subgrade

Typical cross section of a flexible pavement

<u>Asphalt product used in</u> <u>Pavement</u>

- Surface course
 - Binder
 - Tack Coat
 - Prime Coat
 - Seal Coat

Cutback Asphalt

A cutback is produced by dissolving asphalt cement in a lighter hydrocarbon solvent. When the cutback is sprayed on a pavement or mixed with aggregates, the solvent evaporates, leaving the asphalt residue as the binder.

Types and grades of Cutbacks

- *Rapid-Curing (RC).* Produced by adding a light diluent of high volatility (generally gasoline or naphtha) to asphalt cement. These are used primarily for tack coat and surface treatments.
- *Medium-Curing (MC).* Produced by adding a medium diluent of intermediate volatility (generally kerosene) to asphalt cement. These are generally used for prime coat.
- *Slow-Curing (SC).* Produced by adding oils of low volatility (generally diesel or other gas oils) to asphalt cement. They are generally used for prime coat, and as dust palliatives.

Cutback Asphalt



Applying cutback – Prime coat

Emulsified Asphalt

- Emulsified Asphalt is produced by dispersing the asphalt in water as emulsion as shown in the Figure. In this process the asphalt cement is physically broken down into micron-sized globules that are mixed into water containing an emulsifying agent.
- Emulsified asphalts typically consist of about 60% to 70% asphalt cement, 30% to 40% water, and a fraction of a percent of emulsifying agent (basically a soap material).





Types of Emulsified Asphalt

The emulsifying molecule has two distinct components, the head portion, which has an electrostatic charge, and the tail portion, which has a high affinity for asphalt. The charge can be either positive to produce a cationic emulsion or negative to produce an anionic emulsion.

- Anionic emulsions adhere better to aggregate particles with positive surface charges (e.g., limestone)
- Cationic emulsions adhere better to aggregate particles with negative surface charges (e.g., sandstone, quartz, siliceous gravel). Cationic emulsions also work better with wet aggregates and in colder weather

Advantages of emulsified asphalt compared with Cutbacks

Emulsified asphalts are increasingly being used in lieu of cutback asphalts for the following reasons:

- Environmental regulations: Emulsions are relatively pollution free
- Lower application temperature.
- Safety and economy: Emulsions are safe to use more economic.
- Emulsions can also be applied effectively to a damp pavement, whereas dry conditions are required for cutback asphalts

Asphalt cement (binder)

Binder Characterization

- Many tests are available to characterize asphalt cement.
- Since the properties of the asphalt are highly sensitive to temperature, all asphalt tests must be conducted at a specified temperature within very tight tolerances.
- Traditionally the asphalt cement specifications typically were based on measurements of
 - viscosity,
 - penetration,
 - ductility, and
 - softening point temperature.

Penetration and penetration Test

- Asphalt binder consistency or ability to flow can be measured by the value of penetration of a specific device into the binder at a specific temperature. A large penetration value indicates a soft binder and a small penetration value indicates a hard binder.
- Penetration test (ASTM D5). An asphalt sample is prepared and brought to 25°C. A standard needle with a total mass of 100 g is placed on the asphalt surface.



The needle is released and allowed to penetrate the asphalt for 5 seconds, as shown in the Figure. The depth of penetration, in units of 0.1 mm, is recorded and reported as the penetration value.

Absolute and Kinematic Viscosity

 Similar to the penetration, the viscosity measure is used to measure asphalt consistency. Two types of viscosity are commonly measured: absolute and kinematic.

Absolute Viscosity Test

- The absolute viscosity procedure (ASTM D2171) requires heating the asphalt cement and pouring it into a viscometer placed in a water or oil bath at a temperature of 60°C (see Figure).
- The time during which the asphalt flows under pressure between two timing marks on the viscometer is measured using a stopwatch. Then the flow time is used to obtain the absolute viscosity in units of poises.



Kinematic Viscosity Test

- The kinematic viscosity test procedure (ASTM D2170) is similar to that of the absolute viscosity test, except that the test temperature is 135°C. Since the viscosity of the asphalt at 135°C is fairly low, vacuum is not used.
- The time it takes the asphalt to flow between the two timing marks is multiplied by the calibration factor to obtain the kinematic viscosity in units of cenistokes (cSt), (1 stoke = 100 centistokes = 1 cm²/s)

Asphalt Ductility

- The ductility of asphalt is defined as the distance in centimeters, to which it will elongate before breaking
- Ductility is sometimes used as an indirect gage of adhesion and cohesion of asphalt, where Adhesion is the ability to stick to aggregate particles in the asphalt concrete, while Cohesion is the ability to hold the aggregate particles firmly in place

Ductility Test

Ductility test measures the distance in centimeters that a standard briquette of asphalt cement will stretch (@ 5 cm/min at 25 °C) before breaking, as shown in the following figure. It is performed on the asphalt cement samples in accordance with ASTM D113.



Flash Point Test

- The flash point test is a safety test that measures the temperature at which the asphalt flashes.
- The test is done using the Cleveland open cup method (ASTM D92) requires partially filling a standard brass cup with asphalt cement. The asphalt is then heated at a specified rate and a small flame is periodically passed over the surface of the cup, as shown in the Figure.



Cleveland open cup flash point test apparatus.

 The flash point is the temperature of the asphalt when the volatile fumes coming off the sample will sustain a flame for a short period of time. The minimum temperature at which the volatile fumes are sufficient to sustain a flame for an extended period of time is the fire point.

Classification of Asphalt Binder

Asphalt binder is produced in several grades or classes. There are several methods for classifying asphalt binders such as:

- Penetration grading
- Viscosity grading
- Performance grading

Penetration Grading

- Penetration grading's basic assumption is that the less viscous the asphalt, the deeper the needle will penetrate.
- Asphalt binders with high penetration numbers (called "soft") are used for cold climates while asphalt binders with low penetration numbers (called "hard") are used for warm climates.
- Penetration grades are listed as a range of penetration units (one penetration unit = 0.1 mm) such as 60-70.

Penetration grading		Penetration	
system	Grade	min.	max.
Hardest grade	→ 40-50	40	50
Typical grades used in Palestine ——	→ 60–70	60	70
	85-100	85	100
	120-150	120	150
Softest grade. Used for cold climates —	200-300	200	300

Viscosity Grading

Viscosity grading

system

- Based on asphalt binder's viscosity. Viscosity is measured in poise . The lower the number of poises, the lower the viscosity and thus the more easily a substance flows. Thus, AC-5 (viscosity is 500 ± 100 poise at 60° C) is less viscous than AC-40 (viscosity is 4000 ± 800 poise at 60° C).
- Typical asphalt binders used in Palestine are AC-30 and AC-40.
- Viscosity grading is better than penetration grading system.

Grade	Absolute Viscosity (poises)
AC-2.5	250 ± 50
AC-5	500 ± 100
AC-10	1000 ± 200
AC-20	2000 ± 400
AC-30	3000 ± 600
AC-40	4000 ± 800

Performance Grading (PG system)

- The PG system is based on the idea that an asphalt binder's properties should be related to the conditions under which it is used. This involves expected climatic conditions as well as aging considerations.
- Superpave performance grading is reported using two numbers, the first being the average seven-day maximum pavement temperature (in °C) and the second being the minimum pavement design temperature likely to be experienced (in °C).



High Temperature Grades (°C)	Low Temperature Grades (°C)
PG 46	-34, -40, -46
PG 52	-10, -16, -22, -28, -34, -40, -46
PG 58	-16, -22, -28, -34, -40
PG 64	-10, -16, -22, -28, -34, -40
PG 70	-10, -16, -22, -28, -34, -40
PG 76	-10, -16, -22, -28, -34
PG 82	-10, -16, -22, -28, -34