

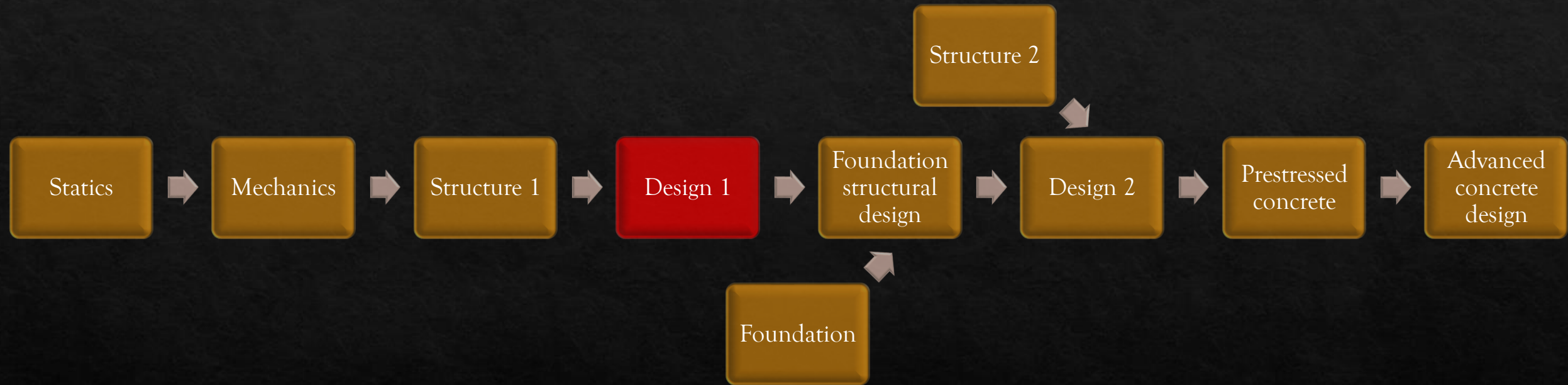
# Reinforced Concrete Design I

## ENCE 335

### Introduction

Dr. Khalil M. Qatu

# You are here !!

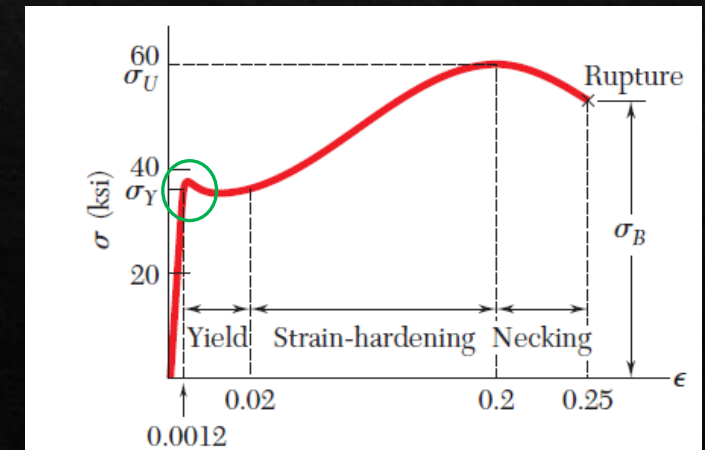
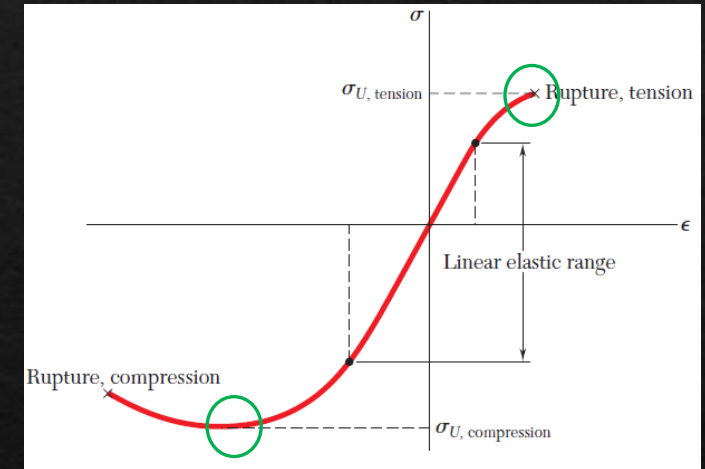
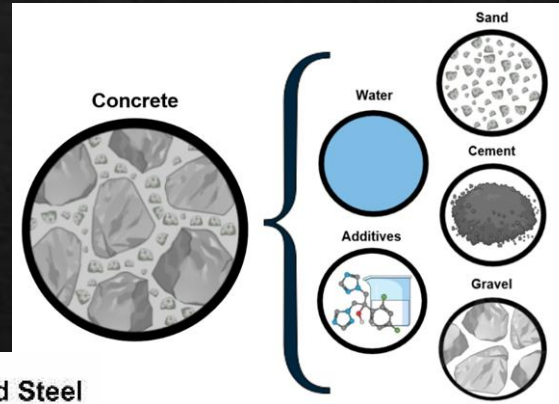


# What is reinforced concrete (RC) ?

- ◇ Concrete is very durable and easy to construct material
- ◇ Relatively cheap
- ◇ High \_\_\_\_\_ Strength
- ◇ Strength range
  - ◇ Normal Strength 10 – 40 MPa
  - ◇ High strength 40-150 MPa
- ◇ BUT ??

- ◇ Steel is more expensive
- ◇ High \_\_\_\_\_ strength

- ◇ Reinforced concrete takes the durability and affordability of concrete and high strength and ductility of steel
- ◇ Location of needed steel reinforcement ??





# What is reinforced concrete ?

Product	ASTM Specification	Designation	Minimum Yield Strength, MPa (psi)	Minimum Tensile Strength, MPa (psi)
Reinforcing bars	A615	Grade 280 Grade 420 Grade 520	280 (40,000) 420 (60,000) 520 (75,000)	420 (60,000) 620 (90,000) 690 (100,000)
	A706	Grade 420	420 (60,000) [540 (78,000) maximum]	550 (80,000) <sup>a</sup>
	A996	Grade 280 Grade 350 Grade 420	280 (40,000) 350 (50,000) 420 (60,000)	420 (60,000) 550 (80,000) 620 (90,000)
	A1035	Grade 690	690 (100,000)	1030 (150,000)

# What is Design ??

◇ Design is the process of **devising** a **system and/or a component** to meet **desired demand**

◇ In Civil Engineering :

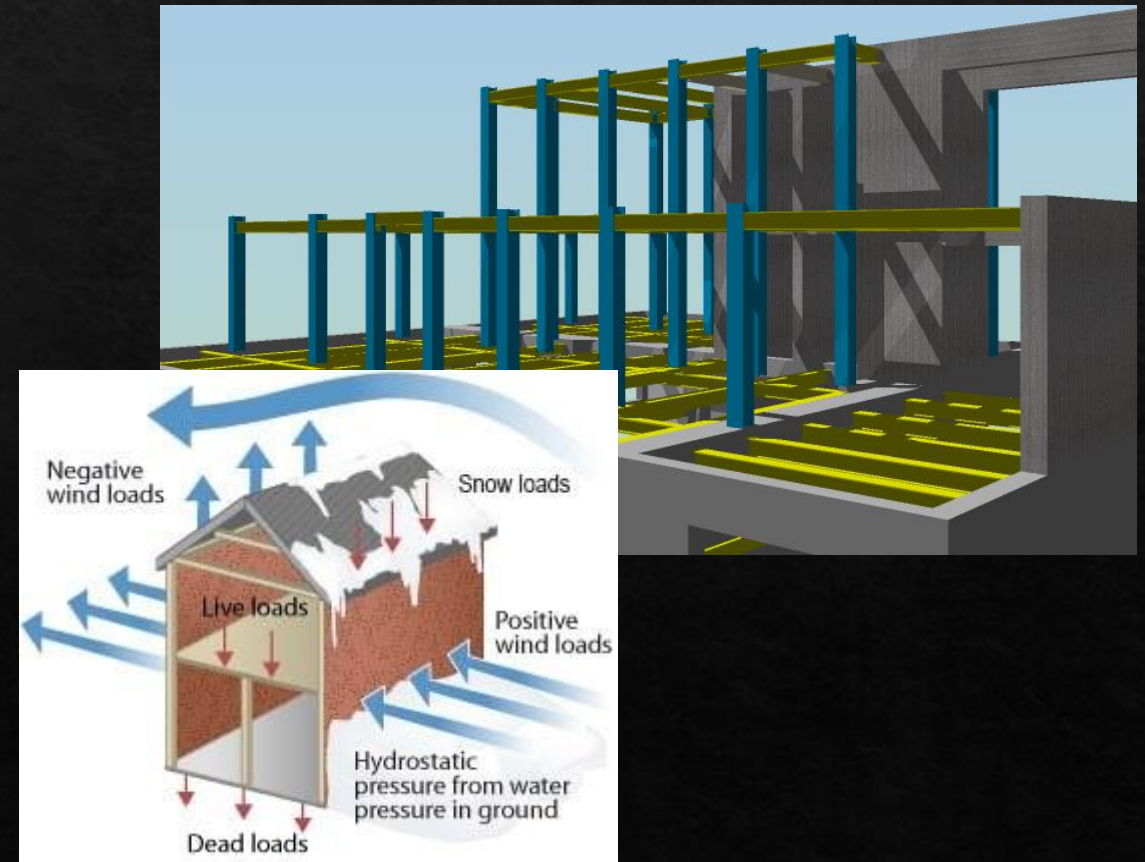
◇ The system:

◇ The demand:

◇ Reinforced Concrete Design ??

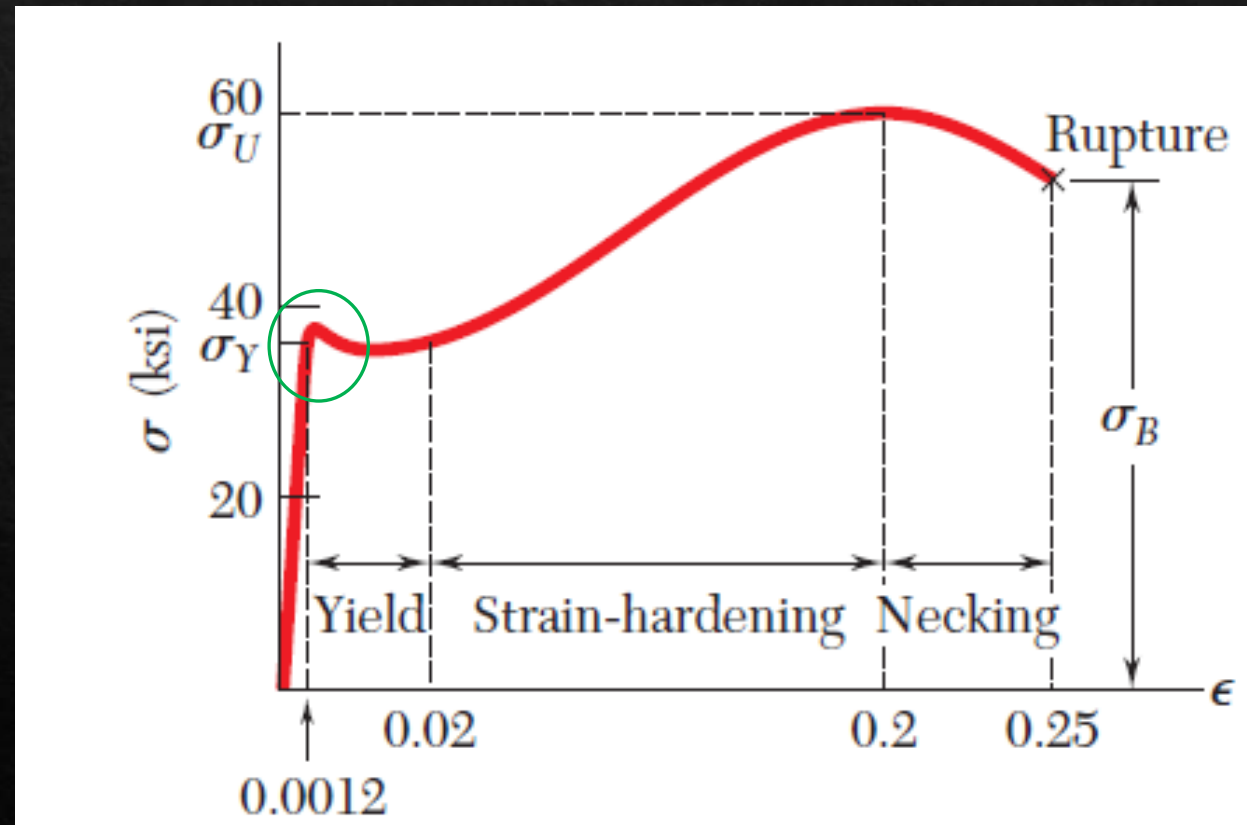
◇ Determine Geometry of concrete member

◇ Amount and location of steel reinforcement



# Design methods

## ◇ Allowable stress design (ASD)

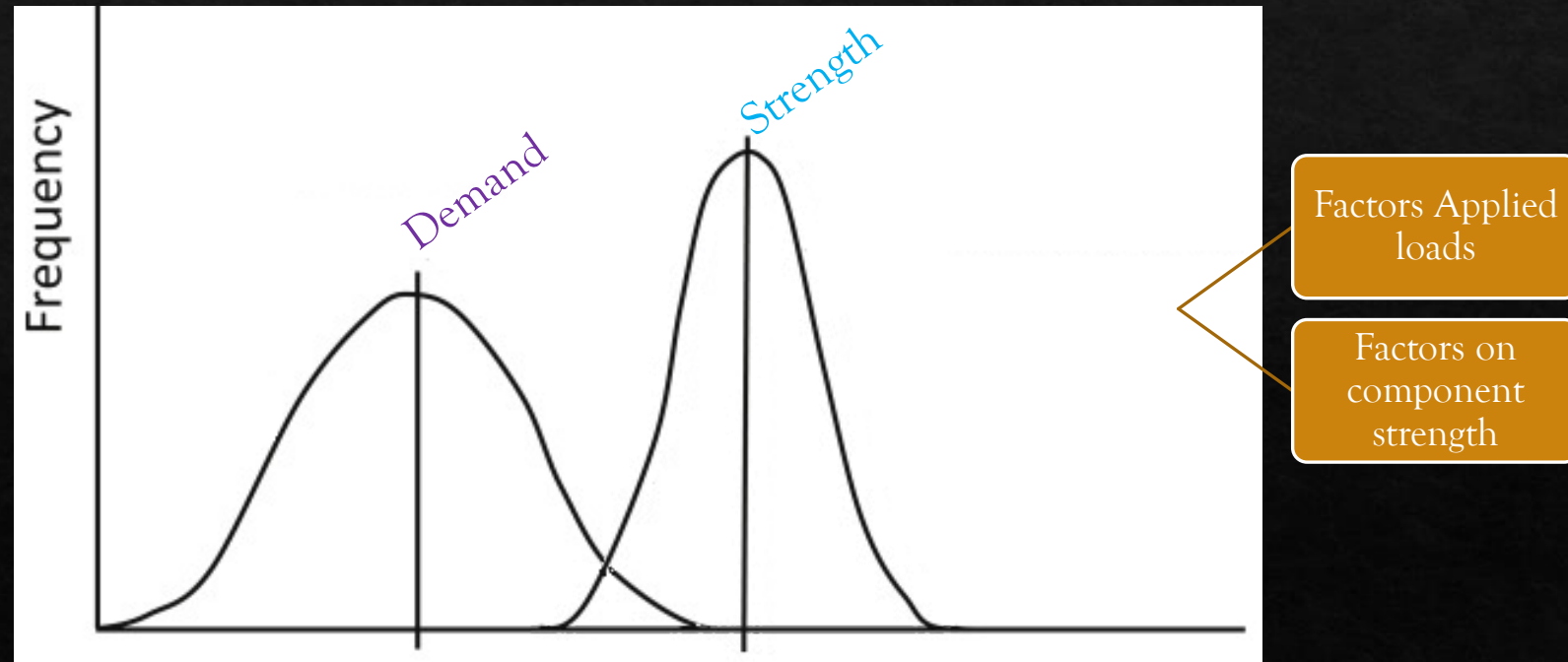




# Design methods

- ◆ Load and resistance factored design (LRFD)

- ◆ This method considers the **variability** in the **applied loads (internal)** and **component strength**



# Design methods

## ◆ Load and resistance factored design (LRFD)

### ◆ Load Factors and load combinations

**Table 5.3.1—Load combinations**

Load combination	Equation	Primary load
$U = 1.4D$	(5.3.1a)	$D$
$U = 1.2D + 1.6L + 0.5(L_r \text{ or } S \text{ or } R)$	(5.3.1b)	$L$
$U = 1.2D + 1.6(L_r \text{ or } S \text{ or } R) + (1.0L \text{ or } 0.5W)$	(5.3.1c)	$L_r \text{ or } S \text{ or } R$
$U = 1.2D + 1.0W + 1.0L + 0.5(L_r \text{ or } S \text{ or } R)$	(5.3.1d)	$W$
$U = 1.2D + 1.0E + 1.0L + 0.2S$	(5.3.1e)	$E$
$U = 0.9D + 1.0W$	(5.3.1f)	$W$
$U = 0.9D + 1.0E$	(5.3.1g)	$E$

Note that factors are higher for loads with high variability

### ◆ After structural analysis we get ultimate internal forces in the member in consideration

$M_u$

$V_u$

$P_u$

Minimum uniformly distributed live loads			
Occupancy or Use	Live Load, kN/m <sup>2</sup>	Occupancy or Use	Live Load, kN/m <sup>2</sup>
Apartments (see residential)		Dining rooms and restaurants	4.8
Access floor systems		Dwellings (see residential)	
Office use	2.4	Fire escapes	4.8
Computer use	4.8	On single-family dwellings only	1.9
Armories and drill rooms	7.2	Garages (passenger cars only)	1.9
Assembly areas and theaters		Trucks and buses <sup>a</sup>	
Fixed seats (fastened to floor)	2.9	Grandstands (see stadium and arena bleachers)	
Lobbies	4.8	Gymnasiums, main floors and balconies <sup>b</sup>	4.8
Movable seats	4.8	Hospitals	
Platforms (assembly)	4.8	Operating rooms, laboratories	2.9
Stage floors	7.2	Patient rooms	1.9
Balconies (exterior)	4.8	Corridors above first floor	3.8
On one and two-family residences only, and not exceeding 9.3 m <sup>2</sup>	2.9	Hotels (see residential)	
Bowling alleys, poolrooms, and similar recreational areas	3.6	Libraries	
Catwalks for maintenance access	1.9	Reading rooms	2.9
Corridors		Stack rooms <sup>c</sup>	7.2
First floor	4.8	Corridors above first floor	3.8
Other floors, same as occupancy served except as indicated		Manufacturing	
Dance halls and ballrooms	4.8	Light	6.0
Decks (patio and roof)		Heavy	12.0
Same as area served, or for the type of occupancy accommodated		Marquees and canopies	3.6
Offices	2.4	Office buildings	
Corridors above first floor	3.8	File and computer rooms shall be designed for heavier loads based on anticipated occupancy	
Penal institutions		Lobbies and first-floor corridors	4.8
Cell blocks	1.9	Schools	
Corridors	4.8	Classrooms	1.9
Residential		Corridors above first floor	3.8
Dwellings (one and two-family)		First-floor corridors	4.8
Uninhabitable attics without storage	0.5	Sidewalks, vehicular driveways, and yards subject to trucking <sup>d</sup>	12.0
Uninhabitable attics with storage	1.0	Stadiums and arenas	
Habitable attics and sleeping areas	1.4	Bleachers <sup>b</sup>	4.8
All other areas except stairs and balconies	1.9	Fixed seats (fastened to floor) <sup>b</sup>	2.9
Hotels and multifamily houses		Stairs and exit ways	4.8
Private rooms and corridors serving them	1.9	One and two-family residences only	1.9
Public rooms and corridors serving them	4.8	Storage areas above ceilings	1.0
Reviewing stands, grandstands, and bleachers <sup>b</sup>		Storage warehouses (shall be designed for heavier loads if required for anticipated storage)	
Roofs		Light	6.0
Ordinary flat, pitched, and curved roofs	1.0	Heavy	12.0
Roofs used for promenade purposes	2.9	Stores	
Roofs used for roof gardens or assembly purpose	4.8	Retail	
Roofs used for other special purposes <sup>e</sup>		First floor	4.8
Awnings and canopies		Upper floors	3.6
Fabric construction supported by a lightweight rigid skeleton structure <sup>f</sup>	0.25	Wholesale, all floors	6.0
All other construction	1.0	Walkways and elevated platforms (other than exitways)	2.9
		Yards and terraces, pedestrians	4.8



# Design methods

## ◆ Load and resistance factored design (LRFD)

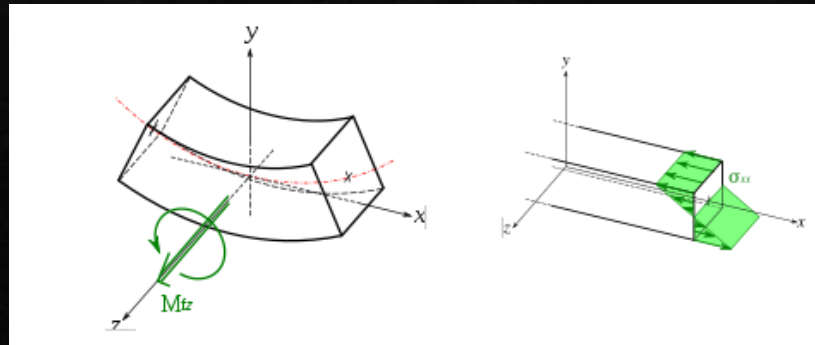
### ◆ Strength reduction factors ( $\phi$ )

$$\phi M_n$$

$$\phi V_n$$

$$\phi P_n$$

- ◆ We get Nominal strength is calculated using concepts from **Statics & Mechanics of Materials**



**Table 21.2.1—Strength reduction factors  $\phi$**

Action or structural element		$\phi$	Exceptions
(a)	Moment, axial force, or combined moment and axial force	0.65 to 0.90 in accordance with 21.2.2	Near ends of pretensioned members where strands are not fully developed, $\phi$ shall be in accordance with 21.2.3.
(b)	Shear	0.75	Additional requirements are given in 21.2.4 for structures designed to resist earthquake effects.
(c)	Torsion	0.75	—
(d)	Bearing	0.65	—
(e)	Post-tensioned anchorage zones	0.85	—
(f)	Brackets and corbels	0.75	—
(g)	Struts, ties, nodal zones, and bearing areas designed in accordance with strut-and-tie method in Chapter 23	0.75	—
(h)	Components of connections of precast members controlled by yielding of steel elements in tension	0.90	—
(i)	Plain concrete elements	0.60	—
(j)	Anchors in concrete elements	0.45 to 0.75 in accordance with Chapter 17	—

# Design CODE

◆ In our region we use the ACI code 318-14 (or 19??)

- ◆ Load Factors
- ◆ Strength requirements
- ◆ Serviceability requirements
- ◆ Reinforcement limits
- ◆ Reinforcement detailing

CHAPTER 3—REFERENCED STANDARDS
CHAPTER 4—STRUCTURAL SYSTEM REQUIREMENTS
CHAPTER 5—LOADS
CHAPTER 6—STRUCTURAL ANALYSIS
CHAPTER 7—ONE-WAY SLABS
CHAPTER 8—TWO-WAY SLABS
CHAPTER 9—BEAMS
CHAPTER 10—COLUMNS
CHAPTER 11—WALLS
CHAPTER 12—DIAPHRAGMS
CHAPTER 13—FOUNDATIONS
CHAPTER 14—PLAIN CONCRETE
CHAPTER 15—BEAM-COLUMN AND SLAB-COLUMN JOINTS
CHAPTER 16—CONNECTIONS BETWEEN MEMBERS
CHAPTER 17—ANCHORING TO CONCRETE
CHAPTER 18—EARTHQUAKE-RESISTANT STRUCTURES
CHAPTER 19—CONCRETE: DESIGN AND DURABILITY REQUIREMENTS
CHAPTER 20—STEEL REINFORCEMENT PROPERTIES, DURABILITY, AND EMBEDMENTS
CHAPTER 21—STRENGTH REDUCTION FACTORS
CHAPTER 22—SECTIONAL STRENGTH
CHAPTER 23—STRUT-AND-TIE MODELS
CHAPTER 24—SERVICEABILITY REQUIREMENTS
CHAPTER 25—REINFORCEMENT DETAILS
CHAPTER 26—CONSTRUCTION DOCUMENTS AND INSPECTION
CHAPTER 27—STRENGTH EVALUATION OF EXISTING STRUCTURES

An ACI Standard and Report

Building Code Requirements  
for Structural Concrete  
(ACI 318-14)

Commentary on  
Building Code Requirements  
for Structural Concrete  
(ACI 318R-14)

Reported by ACI Committee 318

ACI 318-14

