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ENCE 331: Compressibility of soil

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## Previously in Soil Mechanics



# Why shear strength ??

- Soil is made from individual particles (sediments)
- The strength of the soil is contributed to the interaction between these particles
- This interaction takes two forms
  - Cohesion (adhesion between the particles) Depends on type of soil (Clay soils)
  - Friction

Depends on roughness of particles, particle size, and normal stress (Granular soils)

• The forces that break this interaction are shear forces



# Mohr-Coulomb Failure criteria

- The components of shear stress are:
  - Cohesion
  - Friction
- Mohr (1900) presented a theory for rupture in materials that contended that a material fails because of a critical combination of normal stress and shearing stress

$$\tau_f = c' + \sigma' \tan \phi'$$
 OR  $\tau_f = c + \sigma \tan \phi$ 

where c = cohesion

 $\phi$  = angle of internal friction

- $\sigma$  = normal stress on the failure plane
- $\tau_f$  = shear strength



Effective normal stress,  $\sigma'$ 

Table 12.1	Typical Values of Drained Angle of
Friction for	Sands and Silts

Soil type	<b>φ</b> ′ (deg)
Sand: Rounded grains	
Loose	27-30
Medium	30-35
Dense	35-38
Sand: Angular grains	
Loose	30-35
Medium	35-40
Dense	40-45
Gravel with some sand	34-48
Silts	26-35

#### Mohr's Circle and stress transformation

• Since we usually deal with vertical stress (Comp), we need to find the most critical surface (failure plane)



# LAB tests to determine shear strength of soil

- There are many test that can be conducted either in the lab or in the field to determine the soil strength parameters  $(C,\phi)$ 
  - LAB tests
    - Direct Shear Test.
    - Triaxial Compression Test.
    - Unconfined Compression (UCC) Test.
  - Field tests
    - Vane Shear Test.
    - Bore Hole Shear Test.
- Lab tests are more accurate, but require more time and samples will be disturbed to some degree
- Field tests are usually faster, no need to take samples (Undisturbed soil), but it is less accurate.

- The test apparatus is shown
- First the Normal force is applied incrementally so that the sample can consolidate
- The normal stragg is

 $\sigma$  = Normal stress =  $\frac{\text{Normal force}}{\text{Cross-sectional area of the specimen}}$ 

- Then a Horizontal force is applied to the Top half and increased incrementally to failure, and the lateral displacement is measured
- The sheat  $\tau = \text{Shear stress} = \frac{\text{Resisting shear force}}{\text{Cross-sectional area of the specimen}}$



51 mm  $\times$  51 mm or 102 mm  $\times$  102 mm 25 mm (1 in.) high.



- The test is repeated for different specimens of the same soil for different Normal stresses
- Each time the Normal stress and the shear stress at failure are recorded



- General Comments
  - The soil is not allowed to fail along the weakest plane but is forced to fail along the plane of split of the shear box
  - The shear stress distribution over the shear surface of the specimen is not uniform
  - Progressive failure
  - Despite its shortcomings The direct shear test is simple to perform
  - The test can be utilized to determine the interface properties

$$\tau_f = c'_a + \sigma' \tan \delta'$$

where  $c'_{u}$  = adhesion

 $\delta'$  = effective angle of friction between the soil and the foundation meterial



- The triaxial shear test is one of the most reliable methods available for determining shear strength parameters
- Soil specimen about 36 mm (1.4 in.) in diameter and 76 mm (3 in.) long
- The specimen is subjected to a confining pressure by compression of the fluid in the chamber
- The axial load applied by the loading ram and the corresponding axial deformation is measured by a ring or load cell attached to the ram



- The test is performed in two stages
  - Appling the confining pressure  $(\sigma_3)$ 
    - If the drainage is open, then the excess pore water pressure is
    - Depending on the drainage, the test can be Consolidated or Unconsolidated test
  - Appling the deviatoric pressure  $(\Delta \sigma_d)$ 
    - If the drainage is open, then the excess pore water pressure is
    - Depending on the drainage, the test can be Drained or Undrained test
- Depending on the drainage conditions at each stage the following three standard types of triaxial tests generally are conducted:
  - Consolidated-drained test or drained test (CD test)
  - Consolidated-undrained test (CU test)
  - Unconsolidated-undrained test or undrained test (UU test)



- Consolidated-drained test or drained test (CD test)
  - the saturated specimen first is subjected to an all-around confining pressure,  $\sigma_3$ , by compression of the chamber fluid
  - The sample is allowed to consolidate until the pore pressure is completely dissipated
  - Then the deviatoric axial stress is applied incrementally to failure
  - The stress state for any element shown is called .....
  - Total and effective confining stress =  $\sigma_3 = \sigma'_3$
  - Total and effective axial stress at failure  $\sigma_1 = \sigma'_1 = \Delta \sigma_d + \sigma_3$
  - The test is repeated at least two times at different confining pressures
  - The confining pressure and the deviatoric stress at failure are recorded



• Consolidated-drained test or drained test (CD test)



• Consolidated-drained test or drained test (CD test)





• Consolidated-drained test or drained test (CD test) Example:

> The results of two drained triaxial tests on a saturated clay follow: Specimen I:

 $\sigma_3 = 70 \text{ kN/m2}$  $\Delta \sigma_{d,f} = 130 \text{ kN/m2}$ 

Specimen II:

 $\sigma_3 = 160 \text{ kN/m2}$  $\Delta \sigma_{d,f} = 223.5 \text{ kN/m2}$ 

Determine the shear strength parameters.

- Consolidated-Undrained test (CU test)
  - This test is the most common type of triaxial test
  - We can measure the effective soil strength parameters  $(C', \phi')$  and the total stress parameters  $(C, \phi)$
  - Consolidated-drained tests on clay soils take considerable time.
  - Saturated soil specimen is first consolidated by an all-around chamber fluid pressure  $\sigma_3$
  - Then the drainage is closed and the deviatoric axial stress is applied to failure  $\Delta \sigma_{d,f}$
  - This will result in an increase in the pore water pressure  $\Delta u_{d,f}$
  - Major principal stress at failure (total):  $\sigma_1 = \sigma_3 + \Delta \sigma_{d.f}$
  - Major principal stress at failure (effective):  $\sigma'_1 = \sigma_1 \Delta u_{d.f}$
  - Minor principal stress at failure (total):  $\sigma_3$
  - Minor principal stress at failure (effective):  $\sigma'_3 = \sigma_3 \Delta u_{d.f}$





$$\phi = \sin^{-1} \left( \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3} \right) \qquad \phi' = \sin^{-1} \left[ \frac{\sigma_1 - \sigma_3}{\sigma_1 + \sigma_3 - 2(\Delta u_d)_f} \right]$$
Effective stress failure envelope  $\tau_f = \sigma' \tan \phi'$ 
Total stress failure envelope  $\tau_f = \sigma \tan \phi$ 

$$\int \phi'$$

$$\int$$

Shear stress

• Consolidated-Undrained test or drained test (CU test)

• Example:

A specimen of saturated sand was consolidated under an all-around pressure of 105 kN/m2. The axial stress was then increased, and drainage was prevented.

The specimen failed when the axial deviator stress reached 70 kN/m2. The pore water pressure at failure was 50 kN/m2. Determine

- a. Consolidated-undrained angle of shearing resistance,  $\phi$
- b. Drained friction angle,  $\phi'$

- Unconsolidated-Undrained Triaxial Test (UU)
  - This test usually is conducted on clay specimens
  - We can measure the effective soil strength parameters  $(C', \phi')$  and the total stress parameters  $(C, \phi)$
  - Because drainage is not allowed at any stage, the test can be performed quickly
  - Saturated soil specimen is subjected to an all-around chamber fluid pressure  $\sigma_3$  with drainage closed
  - The pore water pressure in the soil specimen will increase by  $u_c$
  - Then the deviatoric axial stress is applied to failure  $\Delta \sigma_{d,f}$
  - This will result in an increase in the pore water pressure  $\Delta u_{d,f}$
  - The total pore water pressure u in the specimen at any stage of deviator stress application  $u = u_c + \Delta u_d$
  - The added axial stress at failure  $\Delta \sigma_{d,f}$  is practically the same regardless of the chamber confining pressure



• Unconsolidated-Undrained Triaxial Test (UU)





- Unconfined Compression Test on Saturated Clay
  - This test is a special type of unconsolidated-undrained test that is commonly used for clay specimens
  - Since the undrained shear strength of clay doesn't depend on the confining pressure  $\sigma_3$
  - The test is performed without any confining pressure  $\sigma_3 = 0$
  - Theoretically, for similar saturated clay specimens, the unconfined compression tests and the unconsolidated-undrained triaxial tests should yield the same values of  $c_u$
  - In practice, however, unconfined compression tests on saturated clays yield slightly lower values  $q_1 q_2 = \frac{\sigma_1}{2}$



